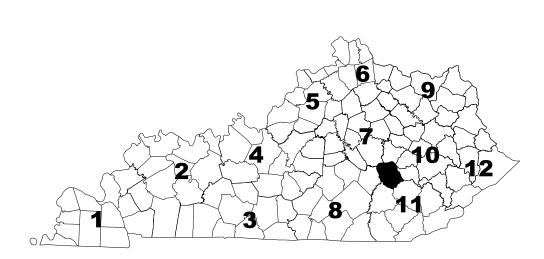
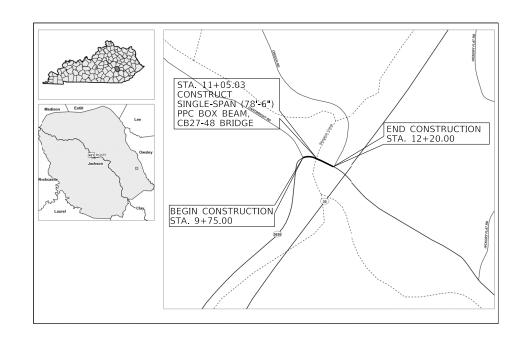


COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

PLANS OF
PROPOSED PROJECT
Jackson County
KY-3630 OVER STURGEON CREEK









LAYOUT MAP

DESIGN CRITERIA

| Major Collector (Rural) | Rolling | N/A | N/A

GEOGRAPHIC COORDINATES

LATITUDE 37 DEGREES 23 MINUTES 50 SECONDS NORTH LONGITUDE 83 DEGREES 50 MINUTES 44 SECONDS WEST

DESIGNED

 % RESTRICTED SD
 N/A

 LEVEL OF SERVICE
 N/A

 MAX. DISTANCE W/O PASSING
 N/A

INDEX OF SHEETS

R01 LAYOUT SHEET
R02 TYPICAL SECTION, GENERAL SUMMARY
AND COORDINATE CONTROL SHEET
LEGEND SHEET
R03 PLAN AND PROFILE SHEET
MAINTENANCE OF TRAFFIC NOTES AND DETOUR PLAN
X1-X6 CROSS SECTIONS
S1-S16 STRUCTURE PLANS
(SEE SI FOR INDEX OF SHEETS FOR BRIDGE)

STANDARD DRAWINGS

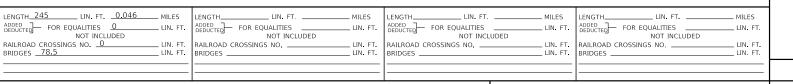
RBI-001-12 RDI-040-01 TPM-175
RBI-002-07 RDX-210-03 TTC-100-05
RBR-001-13 RDX-225-01 TTS-105-02
RBR-005-11 RGX-001-06 TTS-105-02
RBR-010-06 RGX-005-06
RBR-015-06 RGX-100-07
RBR-05-01 RGX-105-09

5: RAILING SYSTEM TYPE II GUARDRAIL TREATMENT EDGELINE RUMBLE STRIPS TWO LANE ROADWAYS EDGELINE RUMBLE STRIPS TWO LANE ROADWAYS NOTES

The contractor is instructed to call 1.300.752.6007 to reach KY 811, the one-call system for information on the location of existing underground utilities. The call is to be placed a minimum of two (2) and no more than ten (10) business days prior to excavation, The contractor should be aware that owners of underground facilities are not required to be members of the KY 811 one-call Before-LPGI (BUD) service, The contractor must coordinate may be necessary for the contractor to contact the County Court Clerk to determine what utility companies have facilities in the area.

THE CONTROL OF ACCESS ON THIS PROJECT SHALL BE BY PERMIT

THIS PROJECT IS OFF THE NH SYSTEM



PROJECT NUMBER: FE02 1100 055 625B00034N (BRIDGE ID 055B00034N)

PROJECT DESCRIPTION: KY-3630 BRIDGE REPLACEMENT OVER STURGEON CREEK

Jared McCommission PE #2866

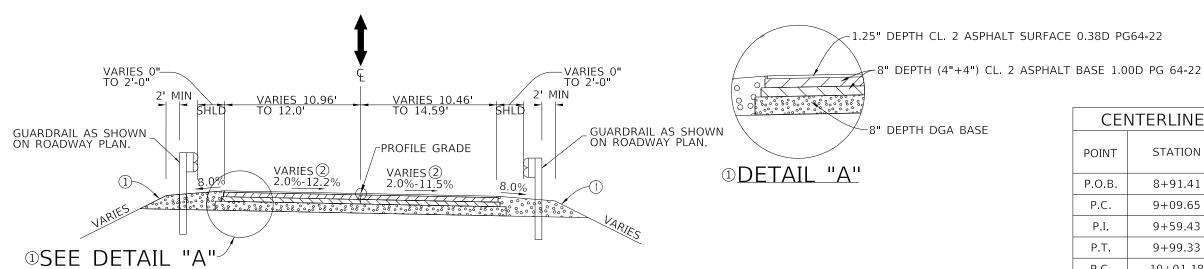
LETTING DATE: 2/20/2025

ITEM NO. COUNTY OF N/A JACKSON

SHEET NO.

OpenRoads Designer v10.12.02.4 USER: JMCCAMMO

FILE NAME: C:\PWWORKING\EAST01\D3739307\055B00034N_R1_LAYOUT.DGN



BRIDGE SECTION

TRAFFIC LANE PAVEMENT ASPHALT SURFACE 1.25" DEPTH CL. 2 ASPHALT SURFACE 0.38D PG64-22 ASPHALT BASE 8" DEPTH (4"+4") CL. 2 ASPHALT BASE 1.00D PG 64-22 DGA BASE 8" DEPTH (4"+4") SHOULDERS SEE BRIDGE LAYOUT SHEET FOR BRIDGE TYPICAL SECTION

NORMAL SECTION

ITEM	DESCRIPTION	UNIT	TOTAL
00020	TRAFFIC BOUND BASE	TON	14
01987	DELINEATOR FOR GUARDRAILL B/W	EACH	6
02200	ROADWAY EXCAVATION	CUYD	60
02223	GRANULAR EMBANKMENT	CUYD	103
02351	GUARDRAIL-STEEL W BEAM-S FACE	LF	162.5
02355	GUARDRAIL-STEEL W BEAM-S FACE A	LF	100
02360	GUARDRAIL TERMINAL SECTION NO 1	EACH	2
02562	TEMPORARY SIGNS (A)	SQFT	180
02569	DEMOBILIZATION	LS	1
02585	EDGE KEY	LF	77
02650	maintain and control traffic ®	LS	1
02726	STAKING	LS	1
08301	REMOVE SUPERSTRUCTURE	LS	1
06515	PAVE STRIPING-PERM PAINT-6 IN	LF	980
03304	bridge overlay approach pavement ${ exttt{1}}$	SQYD	455
20550ND	SAWCUT PAVEMENT	LF	267
21415ND	erosion control ©	LS	1

GENERAL SUMMARY NOTES:

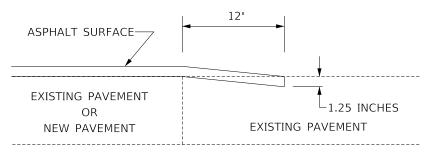
- (A) THIS ITEM INCLUDES ALL TEMPORARY SIGNS USED FOR ROAD CLOSURE AND DETOUR ROUTE.
- B ALL TEMPORARY TRAFFIC CONTROL DEVICES EXCEPT TEMPORARY SIGNAGE ARE CONSIDERED INCIDENTAL TO THIS BID ITEM.
- © SEE SPECIAL NOTE. INCLUDES ALL CLEARING, TEMPORARY EROSION CONTROL BMPS AND PERMANENT SEEDING.

NOTES:

(1) APPROACH PAVEMENT WILL BE PLACED IN ACCORDANCE WITH THE SPECIAL NOTE FOR PLACING BRIDGE OVERLAY APPROACH PAVEMENT. DGA OR OTHER GRANULAR MATERIAL APPROVED BY THE ENGINEER NEEDED FOR SHOULDERS OUTSIDE OF PAVED AREAS WILL BE MEASURED AS GRANULAR EMBANKMENT. FULL-DEPTH PAVEMENT IS ONLY REQUIRED FOR EXCAVATED AREAS FOR CONSTRUCTION AND WIDENED AREAS. THE REMAINDER OF PAVEMENT AREAS SHALL BE WITH LEVELING AND WEDGING AND SURFACING THE ESTIMATED PAVEMENT QUANTITIES ARE SHOWN BELOW: ASPHALT SURFACE: 31 TONS ASPHALT BASE: 34 TONS LEVELING AND WEDGING: 63 TONS DGA BASE: 60 TONS

(2) MATCH EXISTING CROSS SLOPES AT EACH END OF PROJECT AND TRANSITION TO 2.0% CROSS SLOPE ACROSS THE BRIDGE. SEE CROSS SECTIONS.

CENTERLINE COORDINATE DATA STATE PLANE SINGLE ZONE COORDINATES STATION **POINT** EAST (X) NORTH (Y) P.O.B. 8+91.41 3673821.013 5474217.259 5474225.333 P.C. 9+09.65 3673837.368 P.I. 9+59.43 3673882.004 5474247.369 P.T. 9+99.33 3673883.584 5474297.123 P.C. 10+01.183673883.643 5474298.973 P.I. 10 + 39.103673884.847 5474336.871 P.T. 10 + 75.793673869.832 5474371.688 P.I. 11+60.173673836.420 5474449.167 P.O.E. 12+63.83 3673790.904 5474542.299



EDGE KEY DETAIL

KY-3630 JACKSON COUNTY EXISTING BRIDGE ID #055B00034N

COORDINATE SYSTEM COORDINATES FOR HORIZONTAL CONTROL WERE OBTAINED FROM GPS METHODS AND ADJUSTED TO CONTROL OBTAINED FROM KYTC IN RECORD DRAWING 10-279.61_11-278.30 ROADWAY.

BASIS OF ELEVATIONS ELEVATIONS WERE DERIVED FROM GPS METHODS AND ARE ADJUSTED TO THE NAV88 VERTICAL DATUM. GEOID MODEL USED WAS GEOID 12A.

COORDINATE CONTROL POINTS									
DOME	DECCRIPTION	STATE PLANE	SINGLE ZONE (STATION	OFFSET				
POINT DESCRIPTION		NORTH (Y)	EAST (X)	ELEV. (Z)			STATION		
HMB4	1/2" REBAR & CAP	3673867.8930	5474251.1352	966.78	9+50.02	43.26' LT.			
CDI1	5/8" REBAR & CAP	3673814.7222	5474493.5639	963.11	12+09.58	14.14' RT.			
CDI2	5/8" REBAR & CAP	3673648.0660	5474666.1280	979.68	N/A	N/A			



TYPICAL SECTION, GENERAL SUMMARY AND COORDINATE CONTROL SHEET

ITEM NO. COUNTY OF N/A JACKSON

SHEET NO. R2

USER: JMCCAMMO

FILE NAME: C:\PWWORKING\EAST01\D3739307\055B00034N R2 TYPICAL.DGN

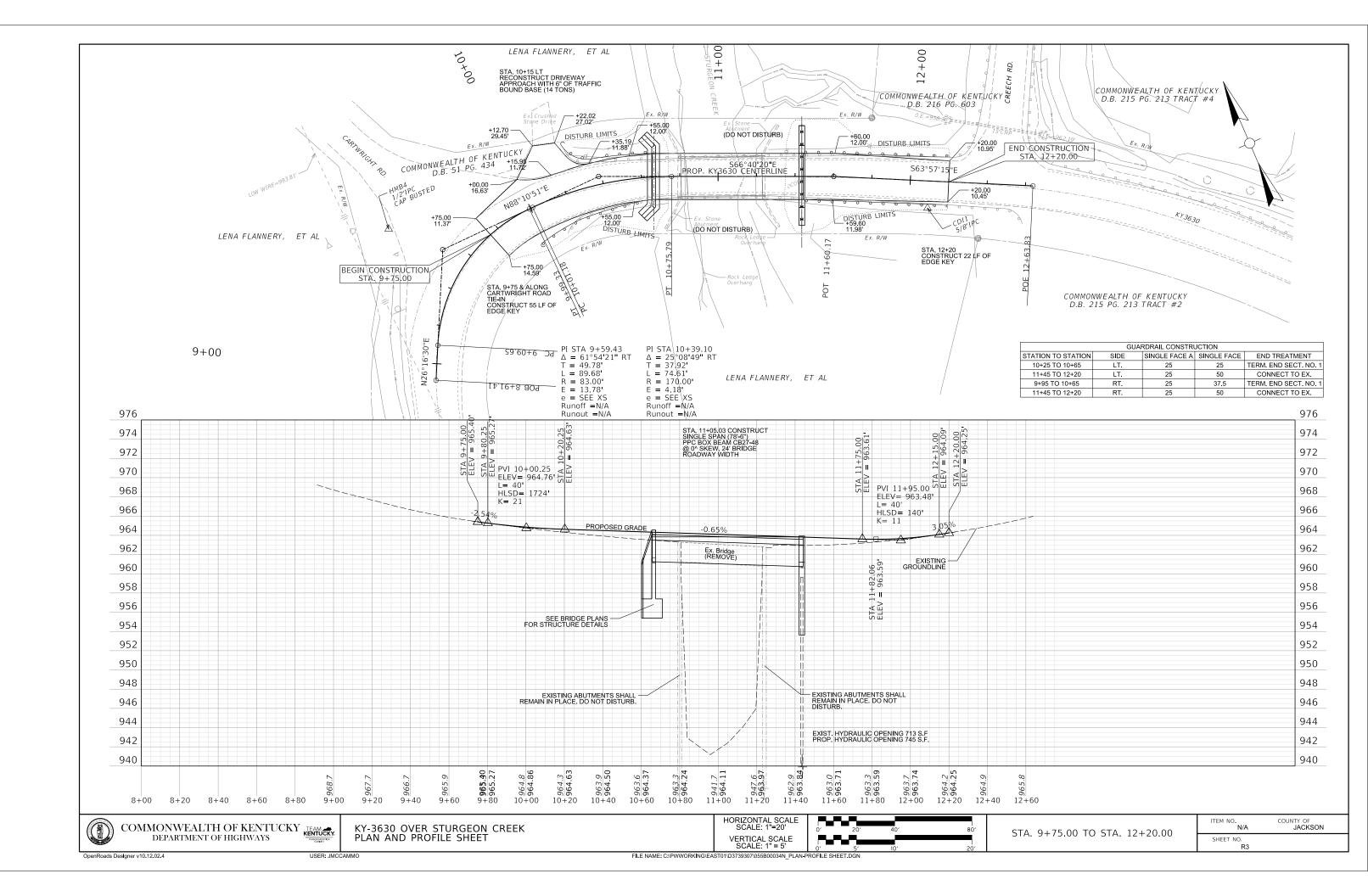
Corporate Limits			Main Water Marker	OWLM		Crash Cushion TY 9		шь	Point (Misc)	_		Telephone Pedestal	[]TEL PED	
County Line	·		Main Water Greater Than 12	OWLMG12		Cross Notch	•NOTCH		Pole	•		Telephone Pole		-0-
Easement			Marker			Curb Box Inlet	C22223		Pole (Light)	¥		Temporary Benchmark		
Fence COA	XX	xx	Sewer Sanitary Marker	OSSM		Curb Notch	•NOTCH		Post	•POST		' , Traffic Light		只
Mineral Parcel Property Line			Sewer Sanitary Force Main	OSANFMM		Combination Pole	=	古	Power Pole	1,037		Traffic Signal		Α.
Right of Way	<i>T.</i>		Marker			Delineator Post	•DP		Quarry	_ %		Control Box	[]jrscв	
Line All Overhead			Sewer Storm Marker	OSTMM		Drop Box	[]		Random (Ground Shot)	+		Traffic Signal Junction Box	ΞŢSJB	
Utility Lines			Multi Utility Bank Marker	OMUBM				P	Railroad Mile Marker	•RRMM		Traffic Signal Pole	•	
Cable Underground Electric With	E (A) OE(A) E (B)	——— Е ———	Oil Line	OOLM		Existing Spring			Railroad Spike	•RRS		Traverse Point	•TRAV	
Quality Levels Duct Underground	E (PA)		Marker	OOLM		Electric Manhole	(<u>EMH</u>)	(EMH)	Right of Way			Tree	\bigcirc	\bigcirc
Electric With Quality Levels	=	====E===E	Steam Line Marker	OSLM		Electric Meter	QEM		Marker	□		TV Junction Box	□TV JB	\bigcirc
Cable Underground	FO (A) OFO(A) FO (B)	FO	Cable Guardrail			Electric Pedestal	[]ELEC PED		R i ght of Way Monument	(•	Utility Pole	•	-0-
Fiber With Quality Levels	FO (CD)	ro	Ditch	——→···	\longrightarrow \longrightarrow \longrightarrow	Electric Pole Electric Junction	•		RR Traffic			Underground		
Cable Underground Telephone With	T (A) OT(A) T (B) T (CD)	тттт	Edge of Water			Box	<u>□</u> EL JB		Signal Pole	•		Storage Tank	(<u>UST</u>)	
Quality Levels	T (PA)		•	~~~		Fire Hydrant	©		RW Parcel		P 000	Utility Test Hole		⊚TH
Duct Underground Telephone With Quality Levels	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	TTT	Fence Hedge			Flag Pole	•FP		Sanitary Cleanout	SSANCO		Water Line Marker	SW LM	
Cable Underground	TV (A) OTV(A)-		Fence Flow Line/Thalweg/	X	x	Force Main Sewer Valve	IXI		Sanitary Manhole	()SANMH	OSANMH	Water Meter	⊗W.M	
TV With Quality Levels	- TV (CD)	TV	Int. Stream or Ditch	<u> </u>		Fuel Tank Inlet	oft I		Satelite Dish	15 <i>D</i>		Water Spigot	ows	
Main Gas With	— GM (A) — OGM(A) — GM (B) —	GM	Guardra il		• • • • • • •	Fuel Tank Vent	OFTV		Septic Tank Cleanout	ostc		Water Valve	SWV	oWV
Quality Levels	— GM (PA) —		Railroad			Gas Meter	OGM		Service Pole	•SP		Water We ll	oww.	
Main Water With Quality Levels	├──	⊢⊢H WM ⊢	Shrub Line			Gas Monitoring			Sewer Air	*37		Yard Light	¥	
Main Water	├		Sink Hole			We ll	ogmw		Release Valve	<u>*</u> I++		Yard Sprinkler	@YS	
Greater Than 12 With Qua l ity Levels	├──	├── 	Tree Line			Gas Valve	≎GV	∘ GV	Shrub			Yard Sprinkler	(13	
Sewer Sanitary With	=	====SAN=====SAN=	Wall (WSM or DSM)			Gas Vent	≎GVE		Sign	◆SIGN		Water Valve	⊕YSWV	
Quality Levels	= = SAN (CD) = = = = = = = = = = = = = = = = = = =	——————————————————————————————————————	Blue Line Stream			Gas Well	≎GW		Sign Post (Single)		Г			
Sewer Sanitary Force Main With	= = SAN FM (A) = OSAN FM(A) = = SAN FM (B) = = = = = = = SAN FM (CD) = = = = =	=====SAN FM======	Lakes and Ponds Regulated Floodway			Guidewires & Anchors		\prec	Sign with 2 posts			<u>Utility</u>	<u>Owners</u>	
Quality Levels	= = SAN FM (PA) = = = = = = = = = = = = = = = = = = =		RDZ Line	, , , , , , , , , , , , , , , , , , ,		Headstone	HEAD STONE		S i gn group (4)	$\overline{b^{o}_{o}d}$		None		
Sewer Storm WIth Quality Levels	=	STORM	ADA Ramp			Interstate Shjeld	(310112)		Station Stamp	STATION		None		
Multi Utility Bank	= = MUB (A) = = OMUB(A) = = MUB (B) = = = = = = = MUB (CD) = = = = =		Anchor Pole	•		Iron Pin	• <i>IP</i>	\sim		STAMP				
Quality Levels	= = MUB (CD) = = = = = = = = = = = = = = = = = = =	MUB	Benchmark	•		Light Pole	¥	¤	Storm Manhole	()SSMH ⊥	Д.			
Oil Line Quality Levels	OIL (A) OOIL(A) OIL (B) OIL (CD)	STM	Bike Lane			Low Wire	+	~	Stub Power	1	呂			
	OIL (PA)		Symbol	040		Mag Nail	•MAG		Stub Telephone		3			
Steam Line Quality Levels	STM (A) OSTM(A) STM (B)	STM	Bollard	•BOLLARD		Mailbox	- In Ad		Survey Cross Notch	•CN				
Cable Underground Electric Marker	OCUGEM		Centerline	+		Manhole	€МН	(EMH)	Survey Curb Notch	•NOTCH				
Duct Underground	O DUGEM		Centerline Stationing	⊙		Mile Marker Post	•MP	(LIIII)	Survey Nail	•MAG				
Electric Marker	ODUGEM		Control Monument			Mineral Parcel	-111 F	M	Survey Sp i ke	•RRS				
Cable Underground Fiber Marker	OCUGFM		Control Point	Δ		Misc Location Point		000	Survey Stone Marker	◆STONE				
Cable Underground Telephone Marker	○CUGTM		Core Hole	OCORE			Omm	•	Swamp	<u>\\</u>				
Duct Underground	○ DUGTM		Crash Cushion TY 6 D	========		Monitoring Well	OMW		Telephone Booth	□JTΒ				
Telephone Marker Cable Underground			Crash Cush i on	====		Parking Meter	••PM		Telephone Junction Box	[]TEL JB				
TV Marker	OCUGTVM		TY 6 A			Pedestrian Signal	©PED SIG		Telephone Line	-•-				
Main Gas Marker	OGLM		Crash Cushion TY 9A			Pins/Pipes	∙IP		Overhead					
	ATTH OF KENTLICKY					PK Nail	•PK		Telephone Manhole	[ĪMH]	TMH	ITEM NO). COU	

COMMONWEALTH OF KENTUCKY KENTUCKY DEPARTMENT OF HIGHWAYS

DRAWING TITLE: LEGEND SHEET

ITEM NO. COUNTY OF N/A JACKSON

SHEET NO. R2A



MAINTENANCE OF TRAFFIC NOTES:

- 1. CONTRACTOR SHALL INVENTORY, REMOVE AND REPLACE EXISTING SIGNS, WITH NEW SIGNS IMPACTED BY THE PROPOSED CONSTRUCTION. OFFSITE SIGNS CONFLICTING WITH THE PROPOSED IMPROVEMENTS SHALL BE REMOVED AND REPLACED AT THE DIRECTION OF KYTC.
- 2. CONTRACTOR SHALL COORDINATE WITH LOCAL RESIDENTS AND OFFICIALS PRIOR TO CONSTRUCTION AND ANY TEMPORARY CLOSURES.
- 3. INGRESS AND EGRESS SHALL BE MAINTAINED TO ALL DWELLINGS AFFECTED BY THE PROJECT.
- 4. CLOSURE SIGNS, DETOUR SIGNS AND BI-DIRECTIONAL LANE CLOSURE SIGNS SHOULD BE PLACED NO SOONER THAN TWO WEEKS PRIOR TO THE CLOSING OF THE BRIDGE (WHEN APPLICABLE) OR PLACING LANE CLOSURES.
- 5. TRAFFIC CONTROL SIGNS AND DEVICES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, THE 2019 KYTC STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AND THE KYTC STANDARD DRAWINGS.
- CONTRARY TO SECTION 106.01, TRAFFIC CONTROL DEVICES USED ON THIS PROJECT MAY BE NEW OR USED IN NEW CONDITION, AT THE BEGINNING OF THE WORK AND MAINTAINED IN LIKE NEW CONDITION UNTIL COMPLETION OF WORK.
- 7. TRAFFIC CONTROL DEVICES SHALL BE PLACED AND MAINTAINED IN THE WORK ZONE IN A MANNER THAT ENSURES THERE IS NO RESTRICTION TO THE VISIBILITY OF APPROACHING TRAFFIC.
- 8. SIGNS NOT APPLICABLE TO CURRENT PHASE OF CONSTRUCTION SHALL BE REMOVED OR COVERED IF LEFT IN PLACE.
- 9. TEMPORARY CLOSURES AND FLAGGERS SHALL BE UTILIZED AS NECESSARY.
- 10. EXISTING KY-3630 BRIDGE AND ROAD SHALL BE CLOSED AND TRAFFIC DIVERTED AND MAINTAINED ON THE TEMPORARY DETOUR DURING CONSTRUCTION OF THE PROPOSED BRIDGE AND ROADWAY.
- 11. BARRICADES SHALL BE TYPE III BARRICADES IN CONFORMANCE WITH MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). SECTION 6F.68
 ADDRESSES TEMPORARY BARRICADES. MINIMUM LENGTH OF TYPE III BARRICADE WILL BE 48", UTILIZE ENOUGH DEVICES OF SUFFICIENT LENGTH
 TO ADEQUATELY BLOCK ROAD USERS FROM EDGE OF ROAD TO EDGE OF ROAD OR CURB TO CURB. IF PROVISIONS HAVE BEEN MADE FOR ACCESS
 OF AUTHORIZED EQUIPMENT AND VEHICLES THE DBT PROJECT TRAFFIC COORDINATOR, OR DESIGNATED REPRESENTATIVE, SHALL ENSURE THAT
 PROPER CLOSURE OF THE ROADWAY IS OBTAINED AT THE END OF EACH WORKDAY.

MAINTENANCE OF TRAFFIC PHASING NOTES:

PHASE

INSTALL AND MAINTAIN TRAFFIC CONTROL SIGNS AND DEVICES IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES TYPICAL APPLICATION 8, FIGURE 6H-8.

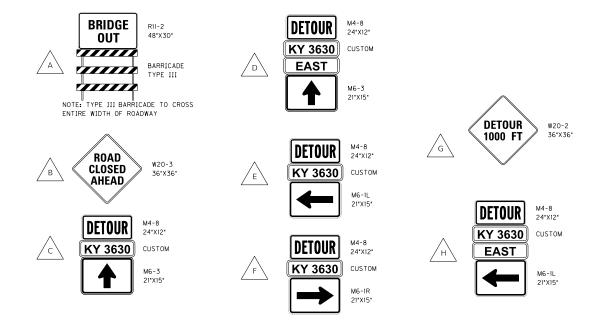
- 2. INSTALL TEMPORARY DETOUR SIGNING AND DEVICES PER THE MOT DETOUR PLAN.
- 3. TYPE III BARRICADES WITH SIGN R11-2 ATTACHED SHALL BE OF SUFFICIENT LENGTH TO CLOSE THE ENTIRE ROADWAY.
- 4. CLOSE THE EXISTING KY-3630 BRIDGE AND ROADWAY AND DIVERT TRAFFIC TO THE TEMPORARY DETOUR AS PER THE MOT DETOUR PLAN. MODIFICATIONS TO THE DETOUR ARE TO BE APPROVED BY THE ENGINEER.

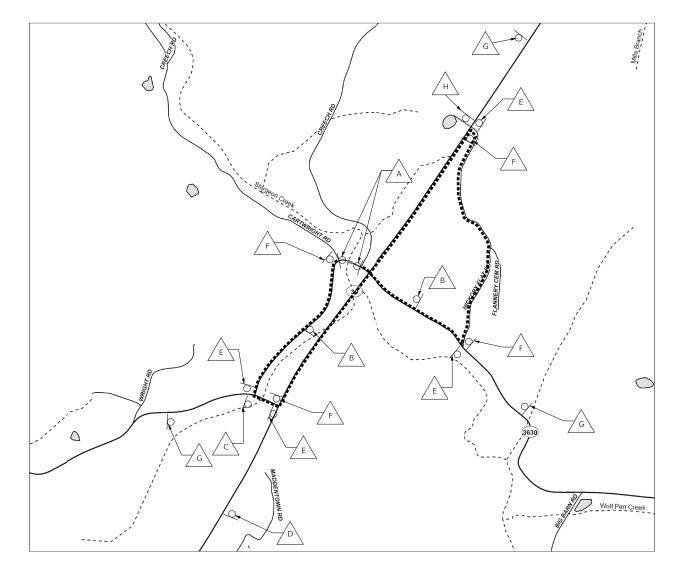
PHASE 2:

- 1. REMOVE THE EXISTING KY-3630 BRIDGE AND ROADWAY.
- 2. CONSTRUCT THE PROPOSED KY-3630 BRIDGE AND ROADWAY.
- 3. INSTALL PERMANENT SIGNS ALONG NEW ROADWAY AND REQUIRED PAVEMENT MARKINGS.

PHASE 3:

- . REMOVE ALL TEMPORARY MAINTENANCE OF TRAFFIC DEVICES AND SIGNS AND TEMPORARY DETOUR SIGNS.
- 2. ROUTE TRAFFIC TO THE PROPOSED KY-3630 BRIDGE AND ROADWAY.





DETOUR PLAN (NTS)

DETOUR ROUTE

DETOUR LENGTH = 2.0 MILES

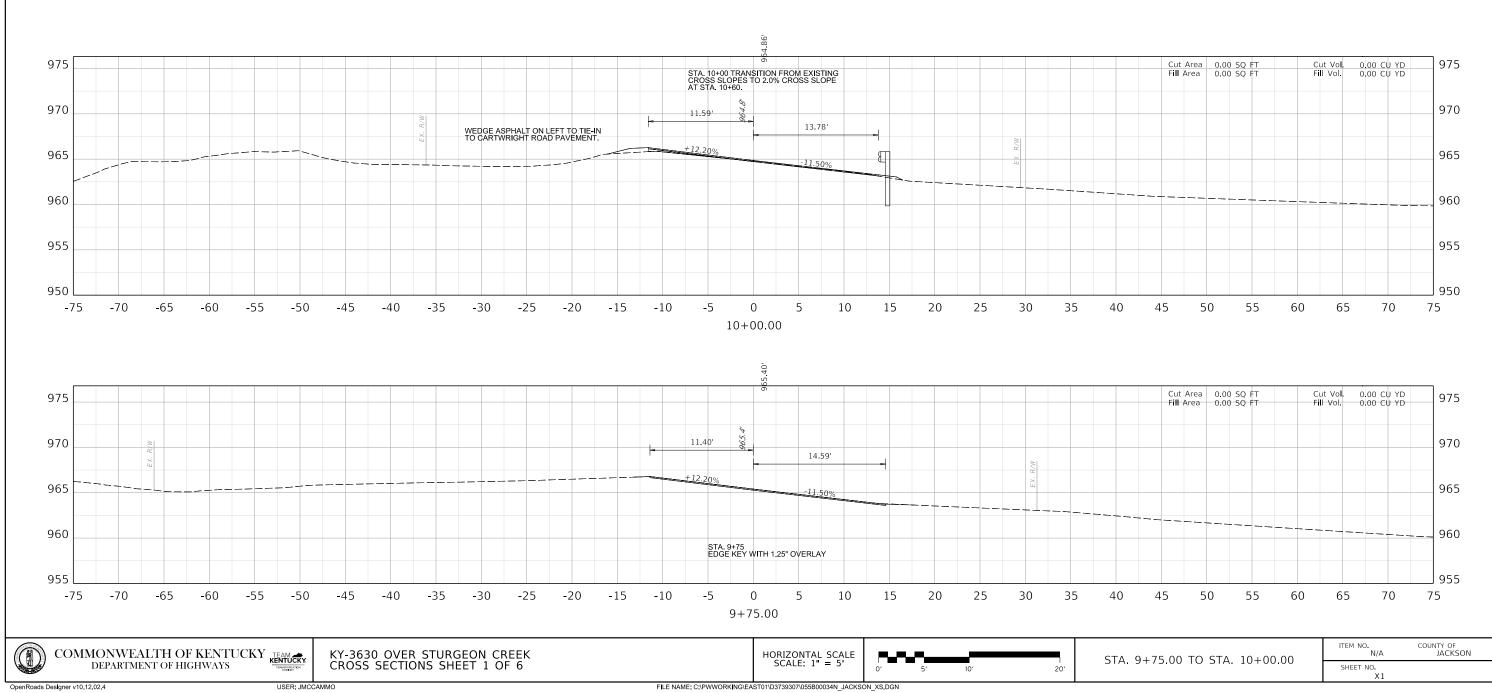
COMMONWEALTH OF KENTUCKY KY-3630 OVER STURGEON CREEK MAINTENANCE OF TRAFFIC NOTES AND DETOUR PLAN

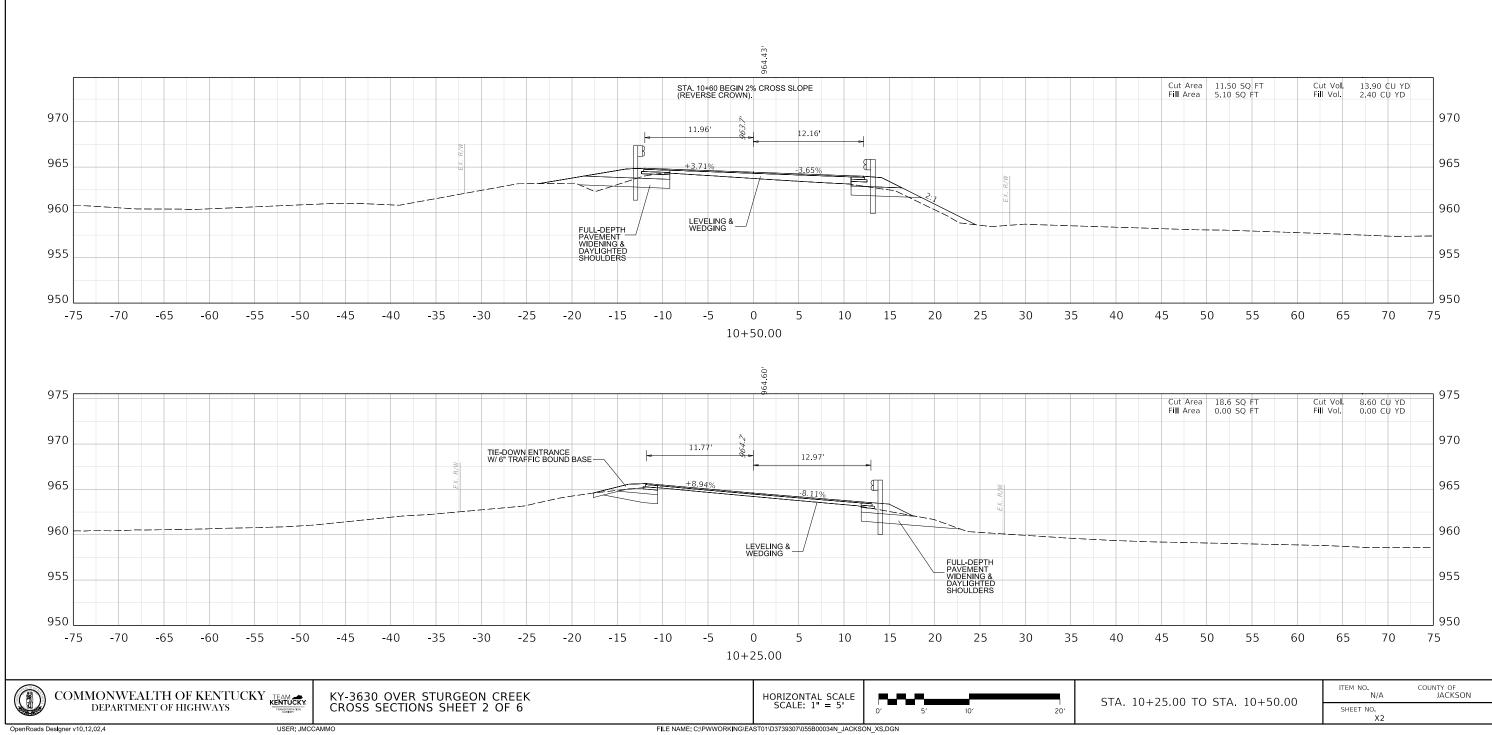
HORIZONTAL SCALE SCALE: NTS 0' xx' xx' xx' xx

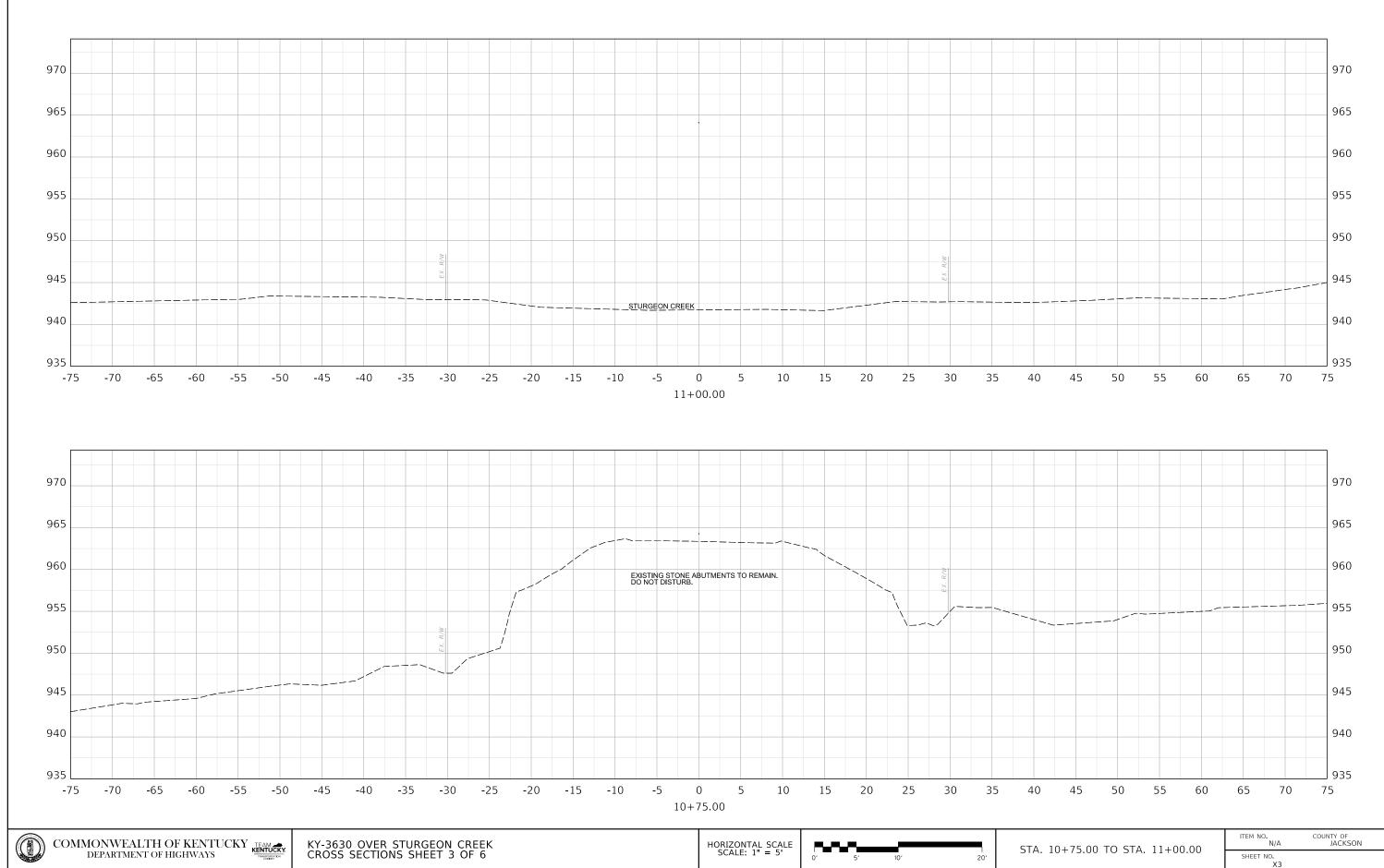
ITEM NO. COUNTY OF N/A JACKSON

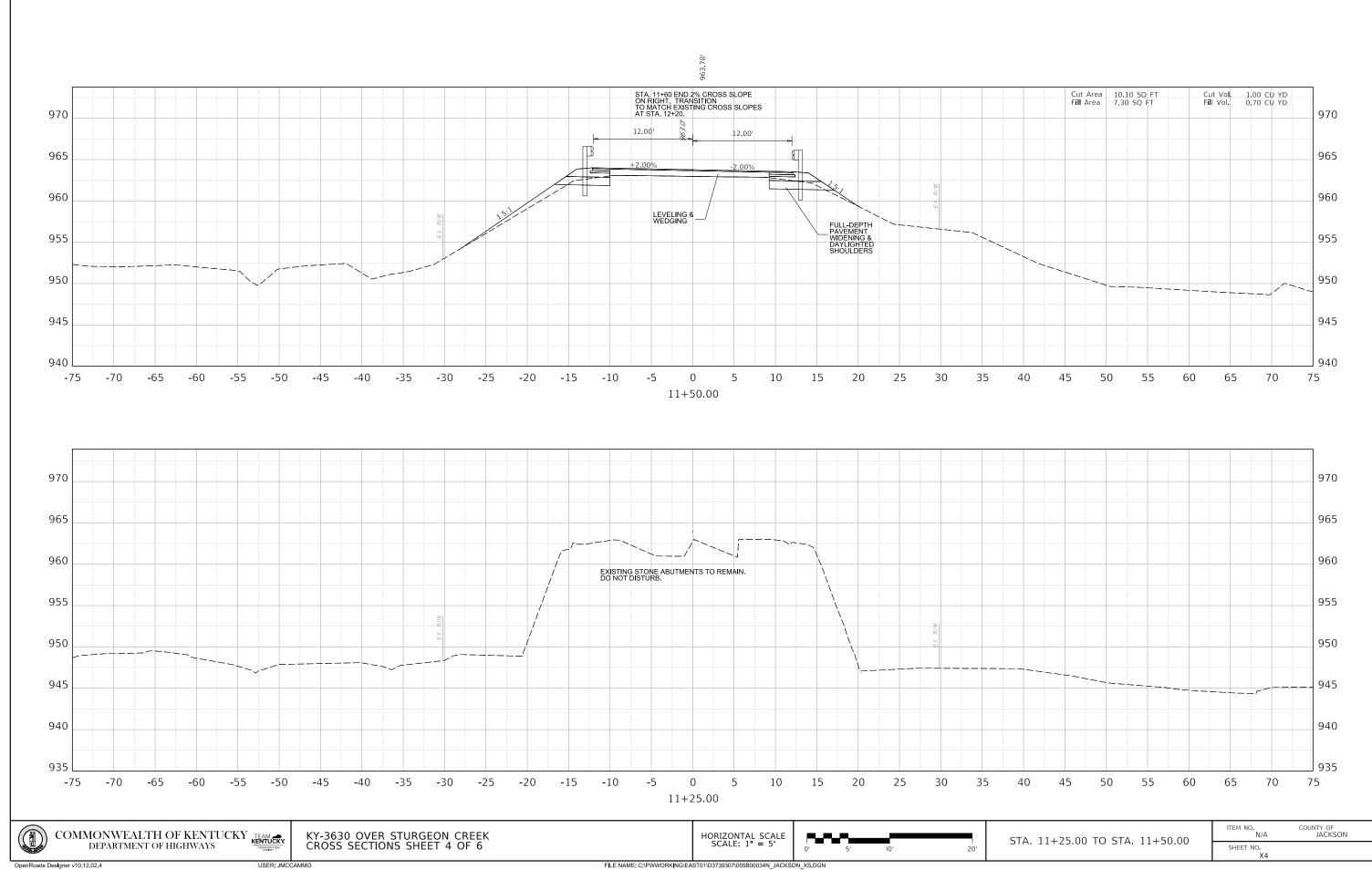
SHEET NO. R04

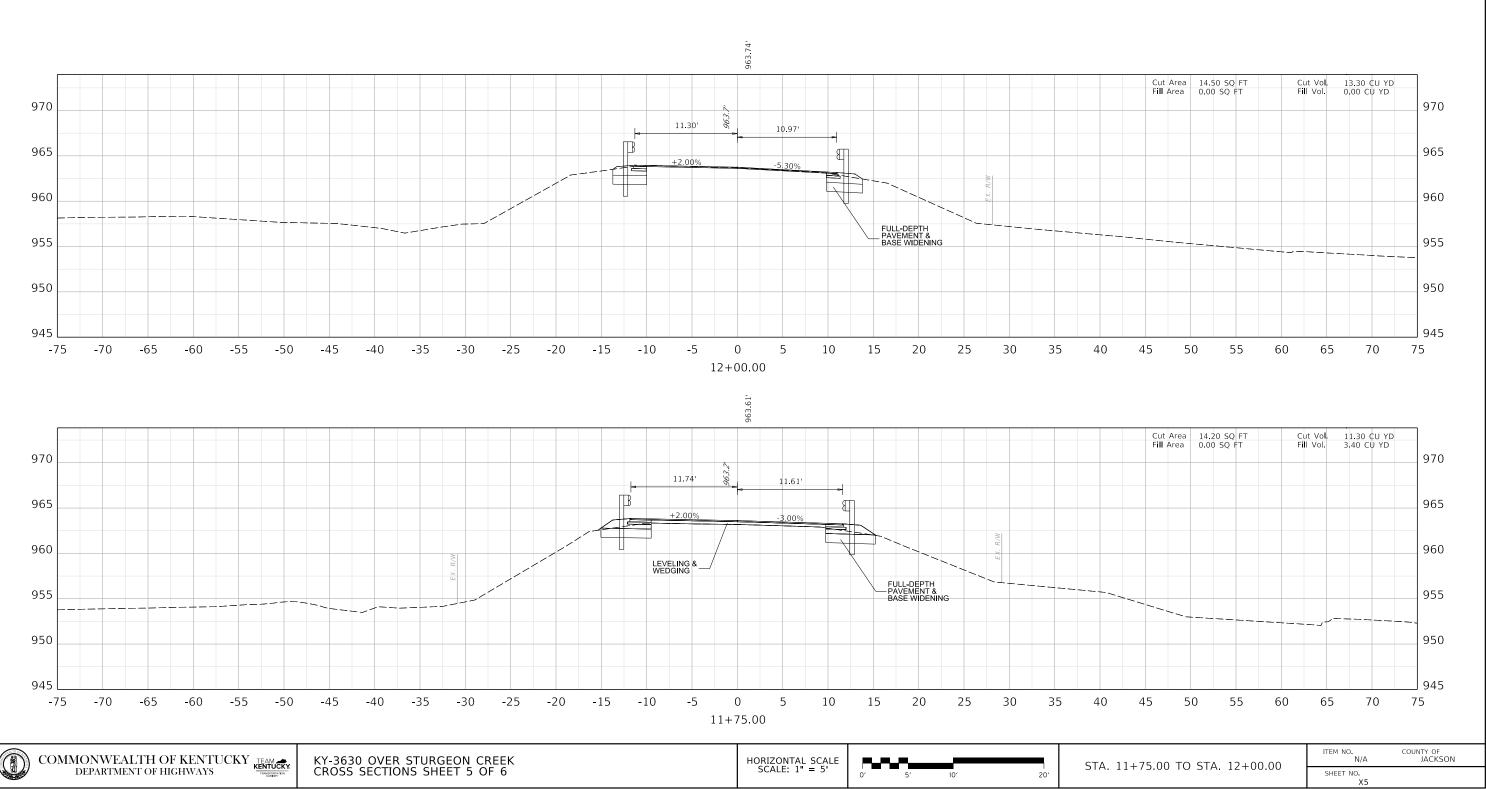
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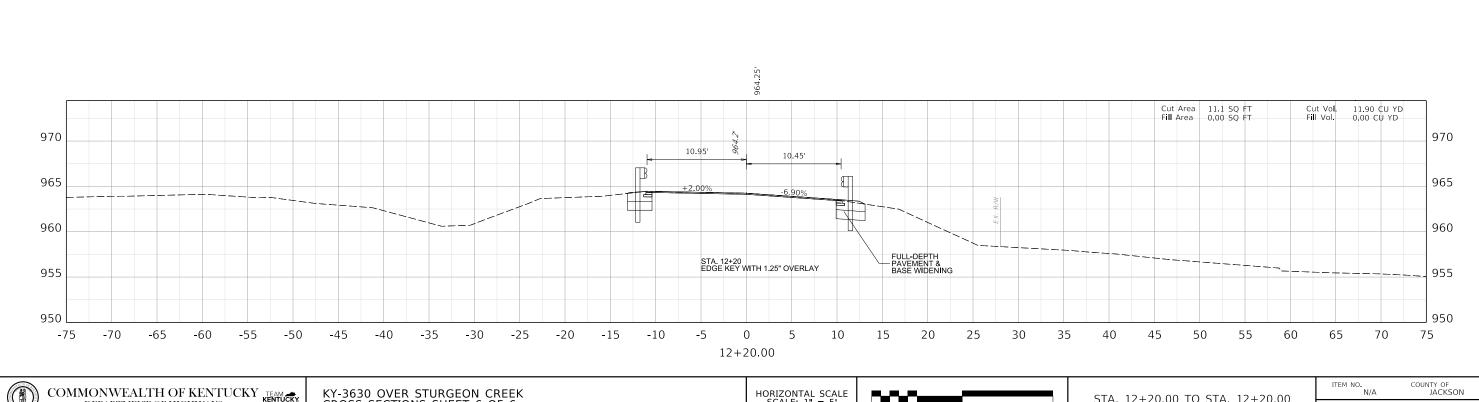












COMMONWEALTH OF KENTUCKY KENTUCKY DEPARTMENT OF HIGHWAYS

KY-3630 OVER STURGEON CREEK CROSS SECTIONS SHEET 6 OF 6

HORIZONTAL SCALE SCALE: 1" = 5'

STA. 12+20.00 TO STA. 12+20.00

SHEET NO. X6

TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

JACKSON COUNTY KY 3630 OVER STURGEON CREEK STA. 11+05.03

	ESTIMATE OF QUANTITIES																				
BID ITEM CODE	08100	08104	08150	08151	08002	02231	23378EC	08664	08051	08033	08003	03299	25099ED	26233EC	08039	08019					
BID ITEM	Concrete Class "A"	Concrete Class "AA"	Steel Reinforcement	Steel Reinforcement, Epoxy Coated	Structure Excavation, Solid Rock	Structure Granular Backfill	Concrete Sealing	PPC Box Beam CB27-48	Piles - Steel HP 14 × 89	Test Piles	Foundation Prep	Armored Edge for Concrete	Deep Beam Bridge Guardrail	Mobilization for Concrete Sealing	Pre-Drilling for Piles	Cyclopean Stone Rip Rap					
UNIT	C.Y.	C.Y.	LBS.	LBS.	C.Y.	C.Y.	S.F.	L.F.	L.F.	L.F.	L.S.	L.F.	L.F.	L.S.	L.F.	TON					
Abutment #1	40.3		3458		31		343														
End Bent #2	33.6	6.6		2950		112	473		133	26					126	264					
<u> </u>																					
Substructu																					
킱																					
sq																					
ิ ช																					
Superstructure		37.2		3958			2308	471				48	146								
BRIDGE TOTALS	73.9	43.8	3458	6908	31	112	3124	471	133	26	1	48	146	1	126	264					

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S1	Title Sheet
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	Layout
	Subsurface Data
	Foundation Layout
	Abutment #1
	Integral End Bent #2
	Box Beam General Notes
	Box Beam CB27-48 Details
	Superstructure Construction Elevations
	Deep Beam Bridge Guardrail
510	Deep Deam Driuge Guarurali
	CDECIAL MOTEO
	SPECIAL NOTES
Special No	te for Concrete Sealing
	SPECIAL PROVISIONS
60.5	SPECIAL PROVISIONS
69 Embanl	SPECIAL PROVISIONS kment at Bridge End Bent Structures
69 Embanl	
	kment at Bridge End Bent Structures
	kment at Bridge End Bent Structures
	STANDARD DRAWINGS
BGX-012-02	STANDARD DRAWINGS Geotechnical Legend
BGX-012-02 BBP-003-02	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams
BGX-012-02 BBP-003-02 BGX-006-10	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams Stencils for Structures
BGX-012-02 BBP-003-02 BGX-006-10 BGX-022	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams Stencils for Structures Joint Waterproofing
BGX-012-02 BBP-003-02 BGX-006-10 BGX-022 BJE-001-14	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams Stencils for Structures Joint Waterproofing Neoprene Expansion Dams and Armored Edges
BGX-012-02 BGX-006-10 BGX-022 BJE-001-14 BPS-011-04	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams Stencils for Structures Joint Waterproofing Neoprene Expansion Dams and Armored Edges HP14x89 Steel Pile
BGX-012-02 BBP-003-02 BGX-006-10 BGX-022 BJE-001-14 BPS-011-04 BDP-002-03	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams Stencils for Structures Joint Waterproofing Neoprene Expansion Dams and Armored Edges HP14x89 Steel Pile Box Beam Bearing Details
BGX-012-02 BBP-003-02 BGX-006-10 BGX-022 BJE-001-14 BPS-011-04 BDP-002-03	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams Stencils for Structures Joint Waterproofing Neoprene Expansion Dams and Armored Edges HP14x89 Steel Pile
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BGX-012-02 BBP-003-02 BGX-006-10 BGX-022 BJE-001-14 BPS-011-04 BDP-002-03 BDP-004-04 BBR-005-11 RBR-001-13	STANDARD DRAWINGS Geotechnical Legend Elastomeric Bearing Pads for Box Beams Stencils for Structures Joint Waterproofing Neoprene Expansion Dams and Armored Edges HP14x89 Steel Pile Box Beam Bearing Details Box Tension Rod Details Guardrail Components Steel Beam Guardrail ("W" Beam) SPECIFICATIONS
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SPECIFICATIONS: All references to the Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction with current Supplemental Specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specs, with interims.

DESIGN LOAD: This bridge is designed for a KYHL-93 live load. The KYHL-93 live load is arrived at by increasing the standard HL-93 truck and lane loads as specified in the AASHTO Specifications by 25%.

FUTURE WEARING SURFACE: This structure is designed for a 15 PSF future wearing surface load.

DESIGN STRESSES: Concrete Class "A" ~ f'c = 3500 psi

Concrete Class "AA" ~ f'c = 4000 psi Steel Reinforcement ~ Fy = 60,000 psi

DESIGN METHOD: All reinforced concrete members are designed by the load and resistance factor method as specified in the current AASHTO Specifications.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Clear distance to face of concrete is 2", unless otherwise noted. Any reinforcement bars designed be suffix (e) in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix (s) in a bill of reinforcement shall be considered a stirrup for purposes of bend diameters.

BEVELED EDGES: Bevel all exposed edges ³/₄" unless otherwise noted.

COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the plans and specifications. Material, labor or construction operations, not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor or anything else required to complete the structure.

SHOP DRAWINGS: Submit shop drawings that are required by the plans and specifications directly to the Division of Structural Design. Is any changes in the design plans are proposed by a fabricator or supplier, submit those changes to the Department through the Contractor.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal dimensions.

SUPERSTRUCTURE SLAB: Ensure the entire superstructure slab is poured continuously, out to out, before allowing any concrete to set.

CONCRETE SEALER: The superstructure deck and overhangs shall be sealed as shown herein these plans. Concrete surfaces (except the deck) shall receive the ordinary surface finish as described in section 601.03.18(A) prior to being sealed.

CONCRETE: Class "AA" is to be used throughout the superstructure. Class "A" is to be used on substructures.

FOUNDATION PREPARATION: Foundation Preparation shall be in accordance with Section 603 fo the Specifications.

TEMPORARY SUPPORTS: Temporary Supports or shoring will not be permitted under beams when pouring the concrete deck slab or when taking top of beam elevations.

GENERAL NOTES

ON-SITE INSPECTION: Each contractor submitting a bid for this work shall make a thorough inspection of the project site prior to submitting a bid and shall be thoroughly familiarized with existing conditions so that work can be expeditiously performed after a contract is awarded. Submission of a bid will be considered evidence of this inspection having been made. Any claims resulting from site conditions will be honored be the Department of Highways.

DIMENSIONS AND ELEVATIONS: All dimensions and elevations given in these plans are based on field surveyed data and dimensions from the old plans. Prior to beginning work or ordering any materials, the contractor shall verify all dimensions and elevations. No claim shall be honored by KYTC regarding site conditions.

MASTIC TAPE: Mastic tape application is required at the abutments as shown in the Joint Waterproofing Detail on sheet S13. See sheet S13 for all mastic tape requirements. The cost of labor, materials, and incidental items for furnishing and installing Mastic Tape shall be considered incidental to the unit price bid for Concrete Class "AA" and no separate measurement or payment shall be made.

TENSION RODS: Tension rods are to be installed prior to pouring the deck.

COFFERDAMS: Cofferdams and/or dewatering methods will be required to facilitate foundation construction.

SHEETING/SHORING: Temporary sheeting and/or shoring may be required for installation of foundations. The contractor shall be responsible for the stability and safety of all excavations.

EMBANKMENTS: Construct the embankments in accordance with Special Provision 69.

SLOPE PROTECTION: Slope protection will be required at the bridge end bent meeting the requirements of Sections 703 & 805 of the Standard Specifications for Road and Bridge Construction, current edition. Place a Class 1 Geotextile Fabric, in accordance with Sections 214 & 843 of the Standard Specifications for Road and Bridge Construction, current edition, between the embankment and the slope protection.

The following abbreviations may have been used in the preparation of these plans:

Bearing

bet. between
b.f. Back Face
BOF Bottom of Footing
BOS Bottom of Slab
bot. Bottom

C to C Center to Center c.e. Current Edition C.Y. Cubic Yards Chd. Chord CL Center Line CIr, Clear Conc. Concrete

CubicCu. DrawingDwg.

Brg.

e.f. Each Face
El. Elevation
eq. Equal
Est. Estimate

ExteriorExt.

F to F Face to Face
f.f. Front Face
f.s. Far Side
fr. Front
ft. Feet
I.D. Inside Diameter

in. Inch

L Left
LBS Low Bridge Seat
LBS, Pounds
M Meter

MPH Miles Per Hour
n.s. Near Side
O.D. Outside Diameter
Opp. Opposite

PC Point of Curvature
Perp. Perpendicular
PI Point of Intersection
PPC Precast Prestressed Concrete

PPCDU Precast Prestressed Deck Unit PSI Pounds per Square Inch

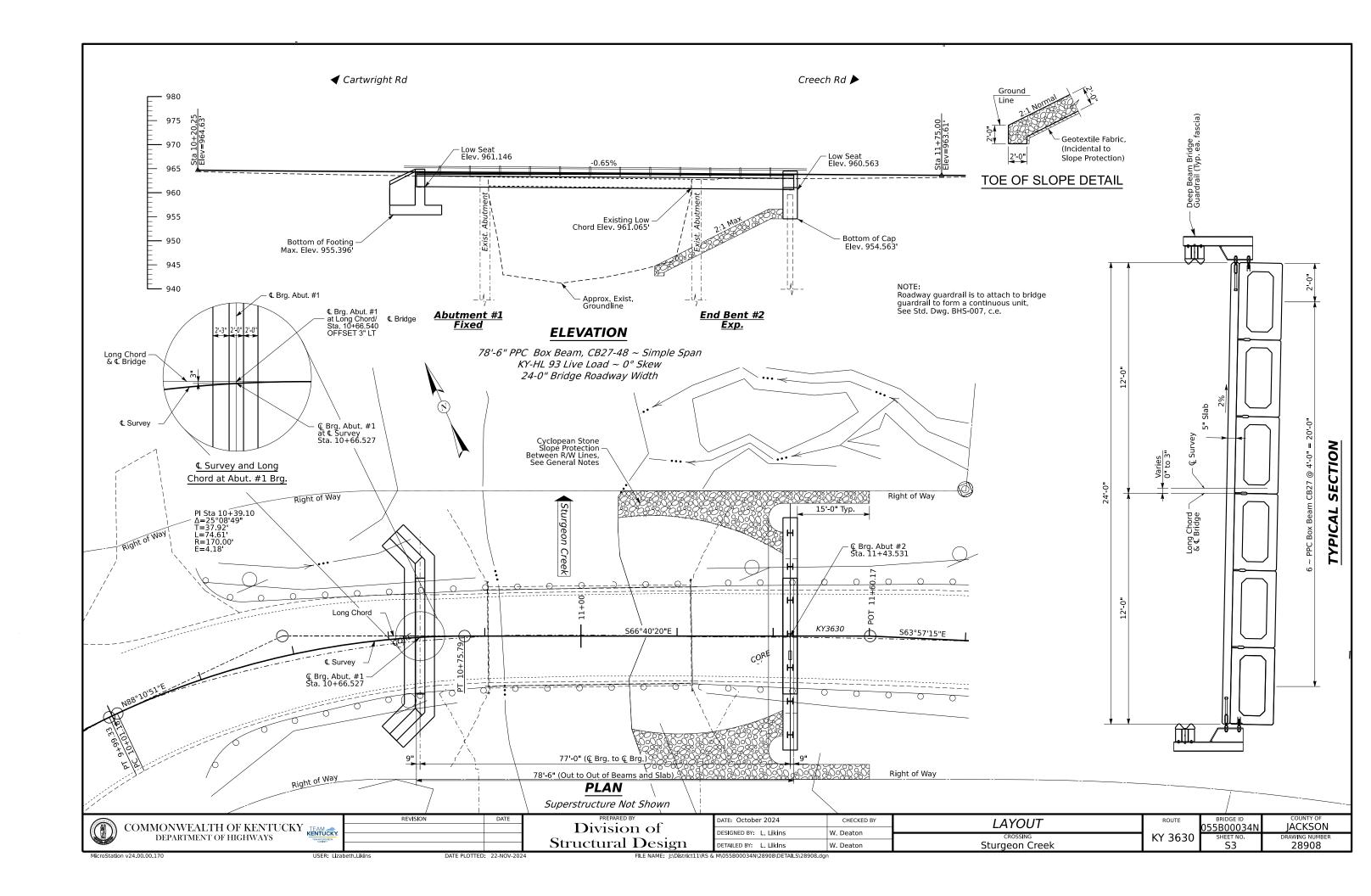
PT Point of Tangency
R Radius

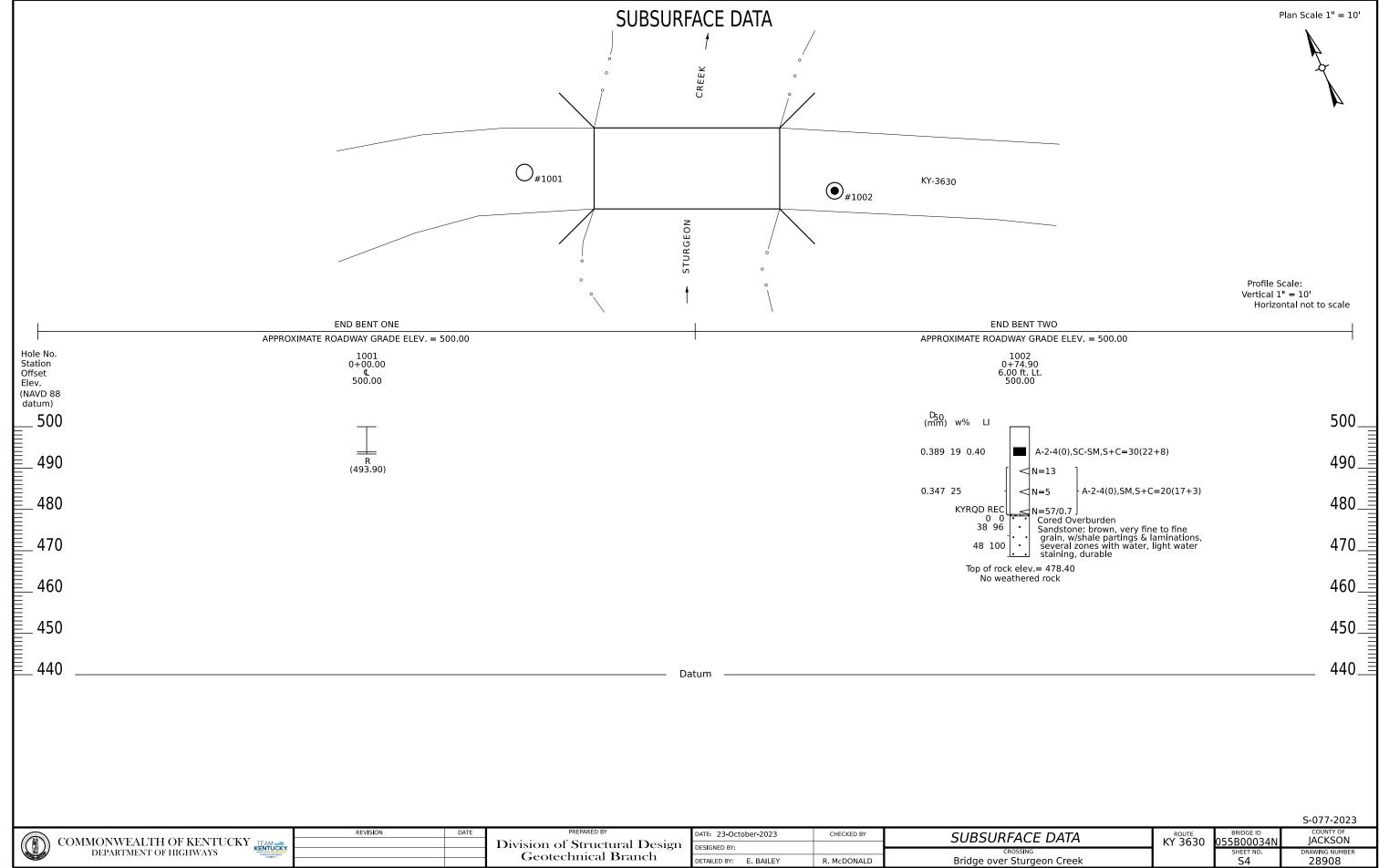
Right

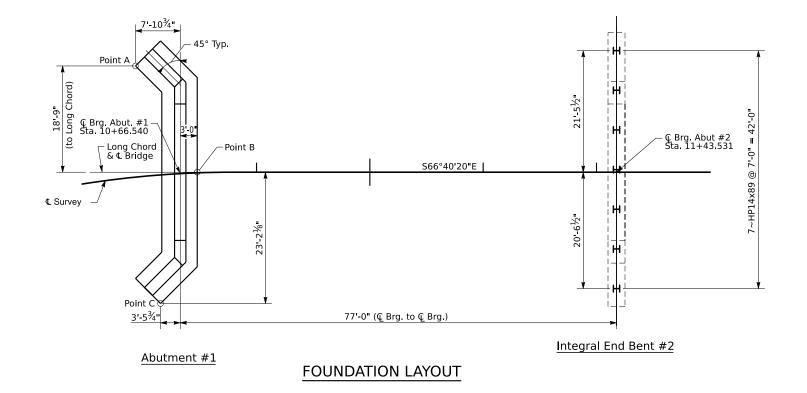
RCBC Reinforced Concrete Box Culvert RCDG Reinforced Concrete Deck Girder

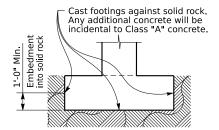
Rea'd Required RR Railroad Shld. Shoulder spa. Spaces Sta Station Std. Standard Str. Straight Tan Tangent Thru Through TOF Top of Footing TOS Top of Slab Tot. Тур. Typical Vert. Vertical W.P. Working Point

REVISION DATE: October 2024 CHECKED BY COMMONWEALTH OF KENTUCKY GENERAL NOTES Division of 55B00034N JACKSON DESIGNED BY: L. Likins W. Deaton KY 3630 DEPARTMENT OF HIGHWAYS Structural Design DETAILED BY: L. LIKINS W. Deaton Sturgeon Creek 28908 DATE PLOTTED: 22-NOV-2024









	read Foo d Abutm	
Point	Plan Footing	As-Built Footing

Point Plan As-Built Footing Elevation

A 955,396' B 955,396' C 955,396'

Footing is designed for a maximum pressure of 8 KSF.

The allowable bearing capacity is 30 KSF.

The Project Resident Engineer is to record the "As-Built Footing Elevation" taken at the bottom of footing and submit one copy of this sheet to:

Kentucky Transportation Cabinet Division of Structural Design Room #322 200 Mero Street Frankfort, KY 40622

If the spread footing foundation is stepped due to unsuitable material found at the given elevation, record the location and elevation of the step as well.

ABUTMENT #1 FOOTING

FOOTING EXCAVATION: All footing excavations in bedrock shall be cut neatly so that no forming or backfilling is necessary in the construction of the portions of the footings located in rock. Concrete should be placed directly against the cut rock faces. Mass concrete should be placed in the excavation from the top of the footing to the bedrock surface where the footing does not extend to the bedrock surface.

The bedrock at this location is highly susceptible to weathering and softening in the presence of water. Water must be kept out of the footing excavations. The footing steel and concrete should be placed the same day as or as soon as practical after the footing excavation is made.

If the bedrock becomes softened at bearing elevations, the softened material should be undercut to unweathered material prior to placing the concrete. Seasonal groundwater fluctuations may cause groundwater infiltration into the footing excavations and a dewatering method may be necessary.

The spread footings shall be embedded a minimum of 1.0 feet into competent unweathered bedrock. Footings may be raised if competent unweathered bedrock is encountered at a higher elevation. Note: Minimum 1.0 feet of embedment must still be attained.

Solid rock excavation will be required for installation of this structure's spread footings.

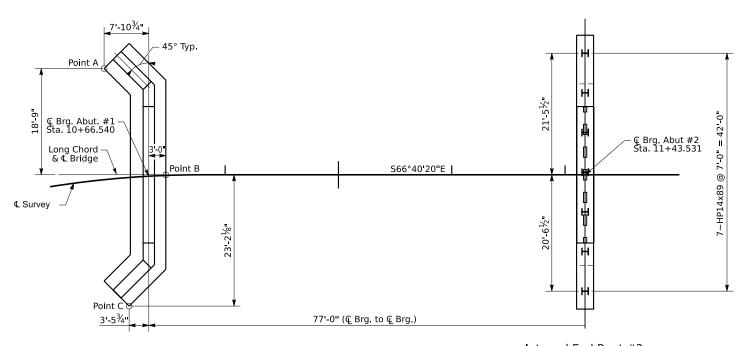
Cofferdams and/or dewatering methods may be required to facilitate foundation construction of spread footings. Include all costs in the price bid for Concrete Class "A".

Construct the embankments in accordance with Special Provision 69.

Temporary sheeting and/or shoring may be required for installation of spread footings. The contractor shall be responsible for the stability and safety of all excavations.

REVISION COMMONWEALTH OF KENTUCKY DATE: October 2024 CHECKED BY **FOUNDATION LAYOUT** Division of **JACKSON** 55B00034N DESIGNED BY: L. Likins W. Deaton DEPARTMENT OF HIGHWAYS KY 3630 Structural Design DETAILED BY: L. LIKINS W. Deaton Sturgeon Creek 28908

ion v24.00.00.170 USER: Lizabeth.Likins DATE PLOTTED: 22-NOV-2024 FILE NAME: J:Oistrict11\RS & M\05580003



Integral End Bent #2

FOUNDATION LAYOUT

~2'-0" dia. Threaded Rod = 1" dia. (Typ.) 6" (Typ.) A563 Nut (Typ.) F436 Washer (Typ.) Threaded Rod Hole = $1\frac{1}{16}$ " dia. (Typ.)

DEFINITIONS OF TERMS

Abutment #1

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished

PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation

PILE TIP ELEVATION AS DRIVEN: Actual point of pile elevation in the

DESIGN AXIAL LOAD: Load carried by each pile as estimated from

CALCULATED FIELD BEARING: Contrary to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to practical refusal.

GENERAL NOTES

SPECIFICATIONS: Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, current edition

MATERIALS: Ensure structural steel piles conform to A.S.T.M. A709 Grade 50, current Specifications.

PAYMENT: Payment for the piles in accordance with plans and specifications will be made at the contract price per linear foot.

PAINT: No painting is required on steel piles.

MILL TEST REPORTS: Furnish mill test reports in triplicate to the

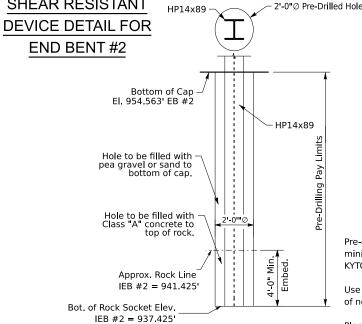
PILE SPLICING: Pile splicing is not permitted on this project.

WITH THREADED ROD DETAIL

PRACTICAL REFUSAL: Drive point bearing piles to practical refusal. For this project minimum blow requirements are reached after total penetration becomes ½ inch or less for 5 consecutive blows, practical refusal is obtained after the pile is struck an additional 5 blows with total penetration of \(\frac{1}{4} \) inch or less. Advance the production piling to the driving resistances specified above and to the depths determined by test pile(s) and subsurface data sheet(s). Immediately cease driving operations if the pile visibly yields or becomes damaged during driving. If hard driving is encountered because of dense strata or an obstruction, such as a boulder before the pile is advanced to the depth anticipated, the Engineer will determine if more blows than the average driving resistance specified for practical refusal is required to further advance the pile. Drive additional

HAMMER CRITERIA: A hammer with a rated energy between 20 and 30 kip-ft will be required to drive the H-piles to practical refusal without encountering excessive blow counts or damaging the piles. The contractor shall submit the proposed pile driving system to the Department for approval prior to the installation of the first pile. Approval of the pile driving system by the Engineer will be subject to satisfactory field performance of the pile driving procedures.

4~Grade 36 Threaded Rods @ 6" = 1'-" (typ. for each row on pile) SHEAR RESISTANT



PRE-DRILLING DETAIL (EB #2)

NON-STRIKE PILE OPTION

As an alternative to striking the pile once placed inside the predrilled hole, the contractor may include shear resistance device on the pile and place and load the piles with an excavator

Details for shear resistance device for piles is provided on this sheet. Use Grade 36 threaded rods with a minimum tensile strength of 58 ksi. The cost of all materials needed is incidental to Pre-Drilling For Piles. The shear resistant device alternative was designed to withstand 125% of the pile's design axial load shown

Provide an excavator with sufficient capacity and reach to lift and place piles without contacting the ground or sides of the boring and to place casing.

Contractor is to ensure hole is cleaned during and after excavation. The rock socket shall be visually inspected. The bottom of hole shall be visible to the inspector by normal means from the surface elevation. If not adequately cleared of debris or water the contractor may be required to clean out the holes using vacuum excavator and/or a pump.

Measure final excavation depths with a weighted tape or other approved methods after final cleaning. Ensure the base of excavation has less than ½ inch of sediment at the time of pile and concrete placement. Do not allow the depth of water to exceed 3 inches during concrete placement

Cofferdams and/or dewatering methods and temporary sheeting and/or shoring may be required to facilitate foundation construction. All costs shall be incidental to Predrilling Piles.

PIL	PILE RECORD FOR POINT BEARING PILES								
Pile No.	Pile Cut-off Elevation	Pile Length In Place	Point of Pile Elevation As Driven	Design Axial Load					
ш	FEET	FEET	FEET	TONS					
		End Bent	#2						
1	959.563			55					
2	959.563			55					
3	959.563			55					
4	959.563			55					
5	959.563			55					
6	959.563			55					
7	959.563			55					

FIELD DATA

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place and Point of Pile Elevations as Driven.

Submit this record to:

Kentucky Transportation Cabinet Division of Structural Design 3rd. Floor East 200 Mero Street Frankfort, KY 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit

PRE-DRILLING PILES

Pre-drilling piles is required for construction of End Bents in order to attain minimum pile embedment as recommended by section GT-605-1 in the KYTC Geotechnical Manual

Use a 24-inch diameter rock socket with a bottom of rock socket elevation of no higher than 937,425

Place pile in center of hole and drive the pile according to criteria on this sheet or use piles with threaded rod details shown on this sheet and apply hydraulic pressure using available excavation equipment to ensure that adequate refusal has been achieved. A temporary casing may be required to prevent collapse of the hole. If used, the casing may be removed as the hole is being backfilled. The predrilled hole shall be filled to top of solid rock with Class "A" concrete conforming to Section 601 of the Standard Specifications: however, provide a mix with a 6 to 10 inch slump at the time of placement. High range water reducing and retarding admixtures and Class F fly ash may be used to obtain this slump. The predrilled hole through overburden shall be backfilled with sand or pea-gravel to bottom of cap.

The cost of all materials, labor, and equipment needed to pre-drill, backfill, and drive pile to refusal shall be included in the price per linear foot for "Pre-drilling for Piles."

Care must be taken that the piling is located correctly since the piling is an integral part of the structure and protrudes up into end bent caps. Contrary to specification for piling, HP14x89 sections shall not vary more than 2" from plan position to prevent any eccentricities in loading. Ensure pile orientation matches that shown. Pile orientation is critical to design

Contractor is to ensure hole is cleaned during and after excavation. The rock socket shall be visually inspected. The bottom of hole shall be visible to the inspector by normal means from the surface elevation. If not adequately cleared of debris or water the contractor may be required to clean out the holes using vacuum excavator and/or a pump.

Measure final excavation depths with a weighted tape or other approved methods after final cleaning. Ensure the base of excavation has less than ½ inch of sediment at the time of pile and concrete placement. Do not allow the depth of water to exceed 3 inches during concrete placement.

Cofferdams and/or dewatering methods and temporary sheeting and/or shoring may be required to facilitate foundation construction. All costs shall be incidental to Predrilling Piles.

PLAN VIEW OF PILE IN ROCK SOCKET

DRIVING CRITERIA

production and test piles if directed by the Engineer.

COMMONWEALTH OF KENTUCKY KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION

Division of Structural Design

DATE: October 2024 CHECKED BY DESIGNED BY: L. Likins W. Deaton DETAILED BY: L. LIKINS W. Deaton

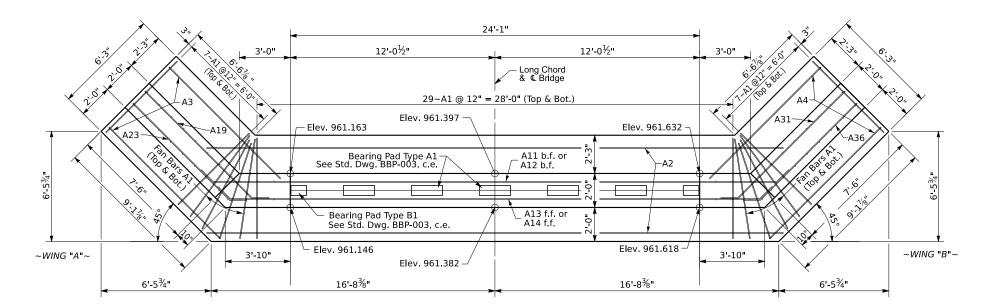
FOUNDATION LAYOUT Sturgeon Creek

55B00034N JACKSON KY 3630

28908

DATE PLOTTED: 22-NOV-2024





PLAN - Top of Wing Mandatory Construction Joint 3~A7 @ 12" = 2'-0" b.f. 3~A7 @ 12" = 2'-0" f.f. 3~A10 @ 12" = 2'-0" b.f. 26~A8 @ 6" = 12'-6" b.f. 23~A9 @ 6" = 11'-0" b.f. 3~A10 @ 12" = 2'-0" f.f. _|12" 12" 10½"_ 13~A8 @ 12" = 12'-0" f.f. 12~A9 @ 12" = 11'-0" f.f. 1.10½" " Sponge Rubber ½" Sponge Rubber A12 b.f. A14 f.f. A7--A10 A9 ¬ A8 ¬ 1/4 N A11 b.f. A13 f.f. 0 0 0 - A2 ~WING "A"~ ~WING "B"~ Bot. of Ftg. El. 955.396 61~A5 @ 6" = 30'-0" b.f. 31~A6 @ 12" = 30'-0" f.f.

ELEVATION

NOTE: Provide 4" weep hole drains in accordance with Subsection 613.03.06.

NOTE: For additional bearing and dowel details see Std Dwg. BBP-002, c.e and BDP-002, c.e.

> Pour footings against solid rock. Any additional concrete required will be incidental to Class "A" Concrete. See General Notes and Foundation Layout for additional requirements.

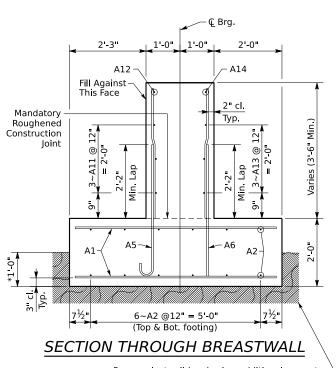
The top portion of the wings are to be constructed after the superstructure is constructed so the concrete can be poured directly against the Sponge Rubber that is against the sides of the beams.

Place $\frac{1}{2}$ " Sponge Rubber on vertical faces as shown. Place 1.25" Sponge Rubber on bearing areas around bearing pads as shown on Sheet S8. Sponge Rubber is incidental to Class "A" Concrete.

Elevations are given at top of concrete.

Construction Sequence:

- 1. Pour Class "A" Concrete below mandatory construction joint.
- 2. Erect beams and install lateral tension rods.
- 3. Pour wings above mandatory construction joint.



Pour against solid rock. Any additional concrete required will be incidental to Class "A" Concrete.

*Footing must be embeded minimum 1'-0" into competent bedrock.

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION C

Division of Structural Design DATE: October 2024 CHECKED BY

DESIGNED BY: L. Likins

W. Deaton

CROSSING

DETAILED BY: M. Bawithawng

L. Likins

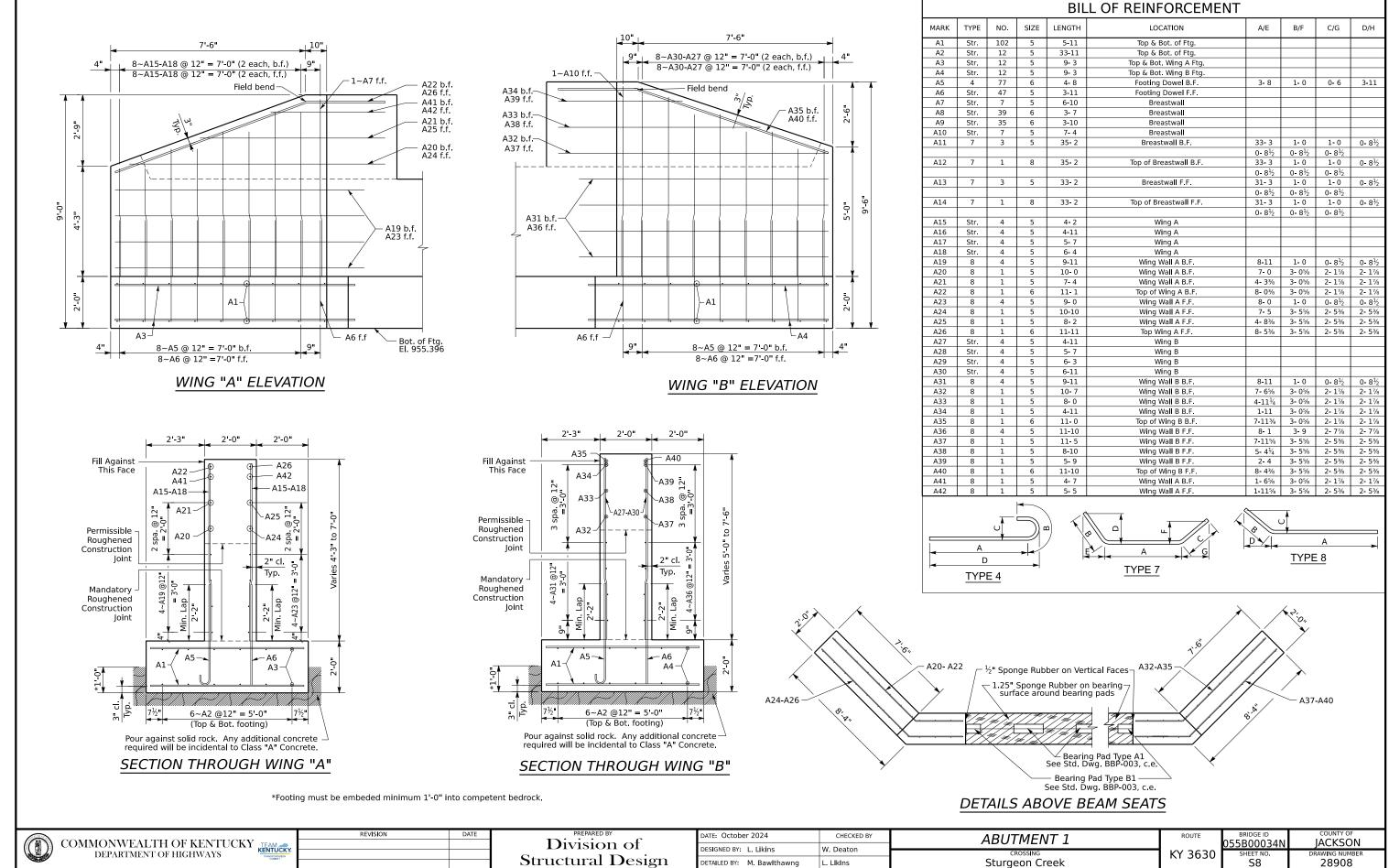
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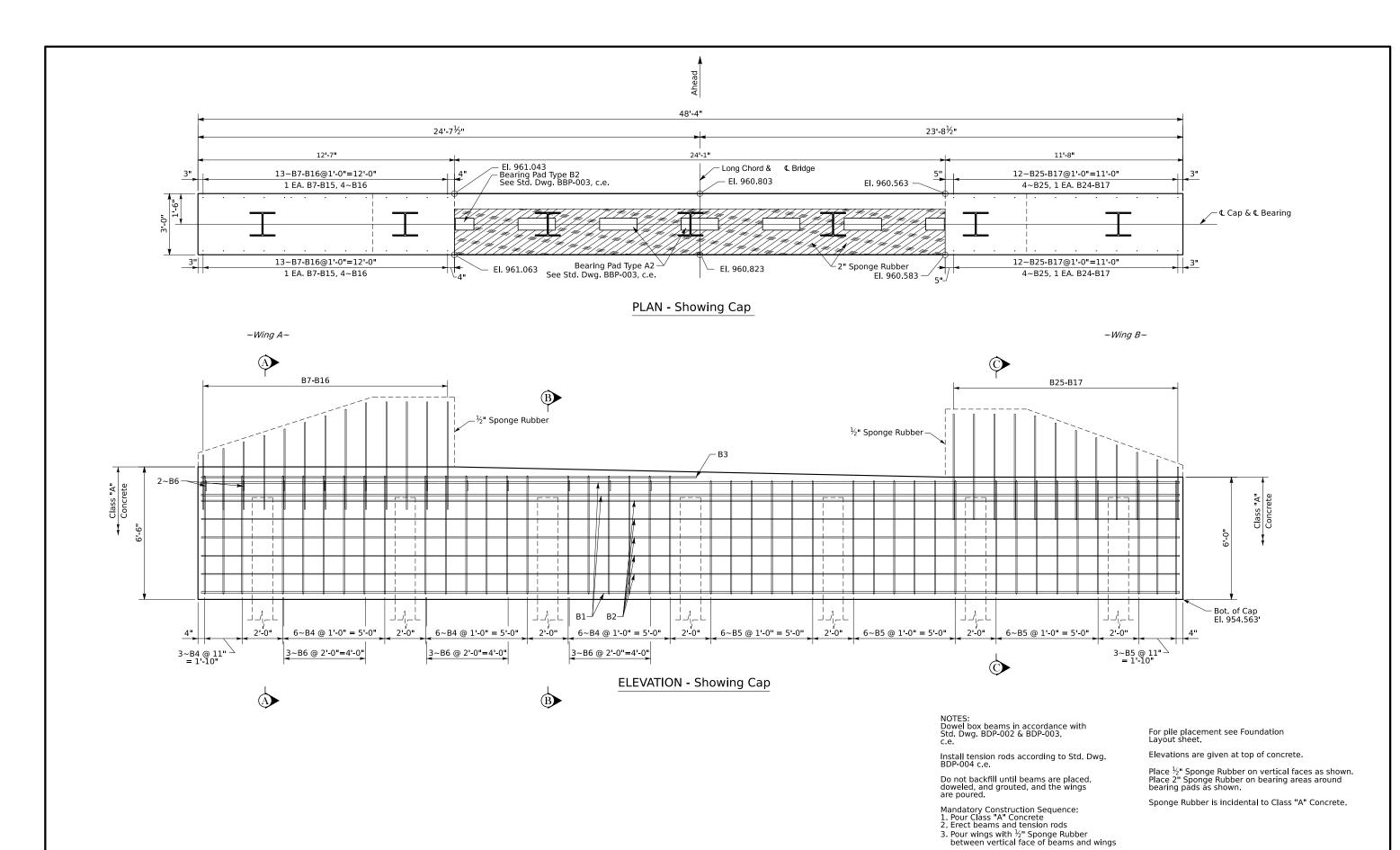
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KY 3630

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SHEET NO. DRAWING NUMBER
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DATE PLOTTED: 22-NOV-2024 FILE NAME: I:\District11\RS & M\055B00034N



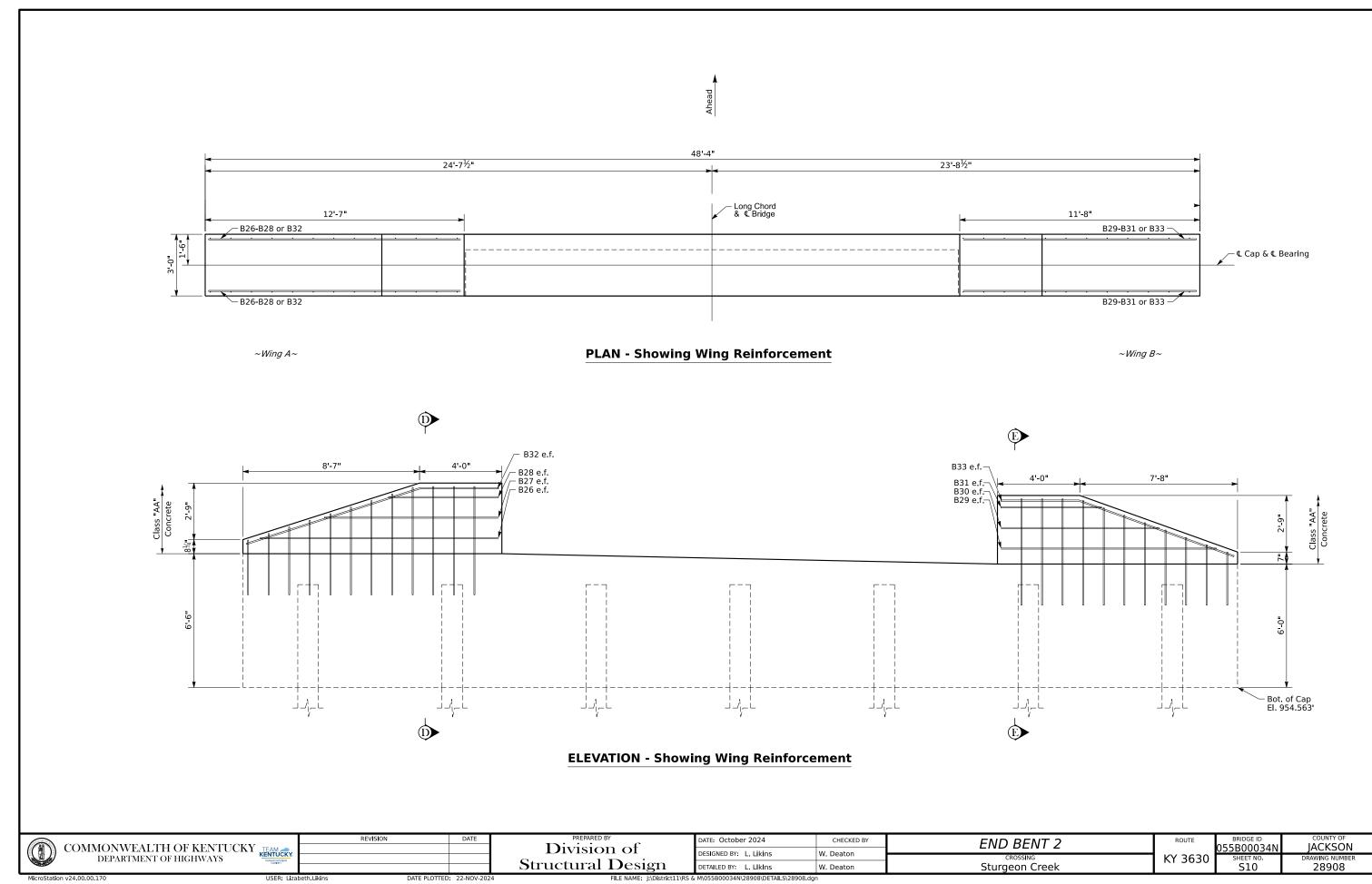


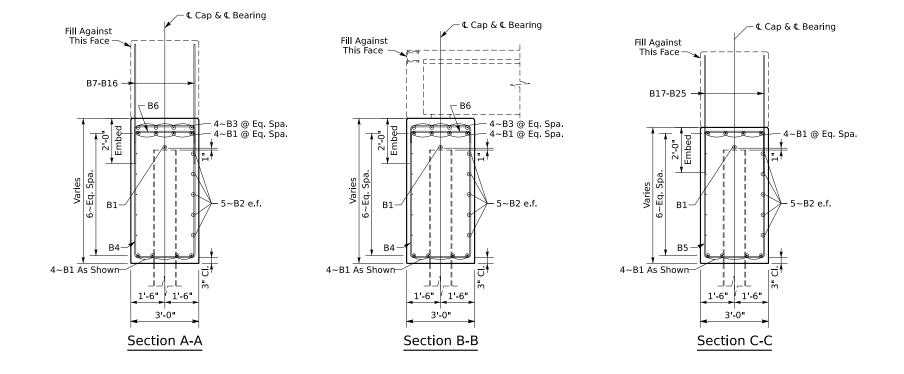
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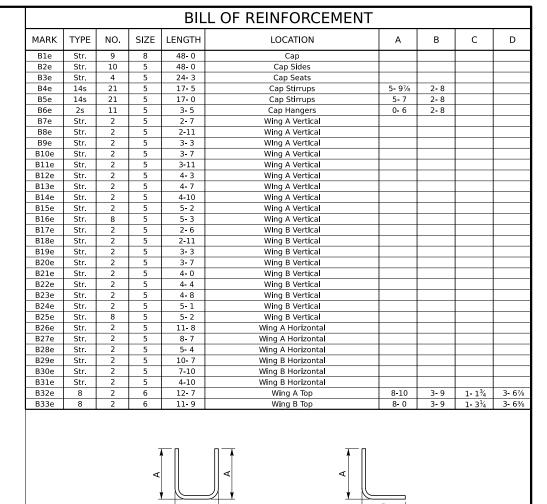
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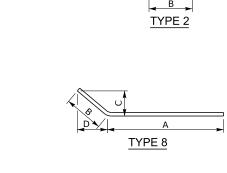
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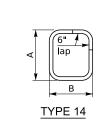
BRIDGE ID 155B000341 END BENT 2 JACKSON DESIGNED BY: L. Likins W. Deaton KY 3630 Sturgeon Creek DETAILED BY: L. LIKINS W. Deaton 28908











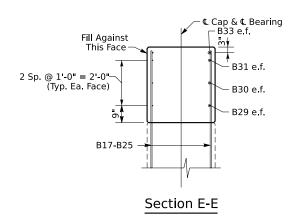
SHEET NO.

JACKSON

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TYPE 5

Fill Against This Face 2 Sp. @ 1'-0" = 2'-0" (Typ. Ea. Face) B7-B16	B32 e.f. B28 e.f. B27 e.f. B26 e.f.
Section	n D-D



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SIGNED BY: L. Likins	W. Deaton	CDOCCING		UDD
TAILED BY: L. LIKINS	W. Deaton	crossing Sturgeon Creek	KY 3630	1

PRECAST PRESTRESSED BOX BEAMS

General Notes

SPECIFICATIONS: All references to the standard Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, with current supplemental specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specifications, with interims.

DESIGN LOADS: Beam sections are designed for 1.25*HL93 (KYHL93) Live Load.

DESIGN LOAD DISTRIBUTION: Contrary to AASHTO LRFD Bridge Design Specifications, the design moment and shear distribution for all beams is 0.5 lanes.

FUTURE WEARING SURFACE: These beams are designed for a 15 PSF future wearing surface load.

SUBSTRUCTURE DESIGN LOADS: Unfactored design reaction forces per beam end.

DC (kips): Beam, Slab (if applicable), and Type II railing dead loads.

DW (kips): Future wearing surface. LL (kips): Beam Live Load reaction per lane x Design load distribution.

LL+I (kips): LL with Dynamic load allowance.

DESIGN DEFLECTIONS:

 Δd (in.): Sum of the downwards deflections caused by the design 5" deck, railing, and future wearing surface. (Positive Downwards)

Δc (in.): Upwards midspan camber of the beam caused by prestressing minus the downward deflection of the beam due to self weight. (Positive Upwards)

MATERIAL DESIGN SPECIFICATIONS:

for Steel Reinforcement FY = 60000 PS. for Prestressed Girder Concrete (Typ. U.N.O.) F'C = 9000 PSIF'CI = 8000 PSI

for Class "AA" Concrete F'C = 4000 PSI for Prestressing Steel F'S = 270000 PSI

DESIGN LENGTH: Beam lengths shown in the Standards represent total beam length. Use the next greater designed section for non-Standard lengths.

CONSTRUCTION METHOD: Transferring bond stress to the concrete will not be allowed, nor releasing of end anchors until the concrete has attained a minimum compressive strength of F'Cl as shown by standard cylinders made and cured identically with the girders; attain F'C at or prior to 28 days. Apply an initial prestress force of 33817 lbs. per low relaxation strand. Beams with honeycomb of such extent as to affect the strength of resistance to deterioration will not be accepted. The allowance of .0005L (length) is made for shortening of beams due to shrinkage and elastic change. Furnish shop plans showing a detensioning plan by numbering, in sequence, the strand pattern.

PRESTRESSING STRANDS: Ensure prestressing strands to be $\frac{1}{2}$ " oversize (0.167 sq. in.) uncoated seven-wire stress relieved, low-relaxation strands conforming to AASHTO M 203, Grade 270. If an alternate strand arrangement or strand type is preferred by the Contractor, the designer that developed the original plans will provide the design and also revise the original plans to reflect the changes. These design and plan modifications will be done at the Contractor's expense.

CORROSION INHIBITOR: Provide a corrosion inhibitor for B-type (non-composite) beams from the list of approved materials.

BEVELED EDGES: Bevel all exposed edges $\frac{3}{4}$ ".

BEAM SEALER: For composite box beams (CB Beams), seal the full length of the exterior face of all exterior beams with the extent from the top of the beam to 1'-0" underneath the beam. For non-composite box beams (B beams), seal all faces of all beams, except take care to ensure the grout pockets are not sealed. Use an approved silane sealer as specified by the Division of Structural Design.

REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of reinforcement. All steel reinforcement is to be epoxy coated in accordance with Section 811.10 of the Specifications. Consider bars marked "C" to be a stirrup for purposes of bend diameters. Non- epoxy reinforcement may be used for fabrication purposes, only, provided that the steel is not used in the top 5 ½" of the beam and the location of the steel is indicated on the shop drawings.

FABRICATION: Beams shall not be fabricated more than 120 days before the deck is to be poured.

GROUT: Provide non-shrink grout for anchor dowels, shear keys, and tensioning rod blockouts conforming with Section 601.03.03 of the Specifications. When side by side superstructure is utilized, grouting will be completed after lateral tension rods have been fully tightened and before leveling devices have been removed. Include the cost of furnishing and placing grout in the price of beam.

RAILING SYSTEM TYPE II: Furnish this material per these specifications.

THEM	DESCRIPTION	MATERIAL SPECIFICATION	COATING SPECIFICATION
Post	W6x25	ASTM A36 or A572	A123
Channel	C7x9.8	ASTM A36 or A572	A123
Plate	½"x 7"	ASTM A36 or A572	A123
Tubing	8x4x0.1875	ASTM A500 or A501	A123
Bolts	5/ ₈ II	ASTM 4307	A153
Nuts	for 5/8"	ASTM A563 Grade A or better	A153
Washers	for 5/8"	ASTM A563, Grade A or better	A153
Stud	11/4"	ASTM A108 (1045 C.D. Bar)	B633, Type II, Class 25
Ferrule	2½"x 5"	ASTM A108 (11L17 Steel)	B633, Type II, Class 25
Wire	3/8"	ASTM A510 (1018 Steel)	B633, Type II, Class 25
Nut _	for 1 $\frac{1}{4}$ " Bolt	ASTM A108 (12L14 Steel)	B633, Type II, Class 25
Nut	for 1 $\frac{1}{4}$ " Stud	ASTM A325M	B633, Type H, Class 25
Washers	for 1 $\frac{1}{4}$ " Stud	ASTM A325M	B633, Type II, Class 25

Use the current edition of the references listed below with these standards.

STANDARD DRAWINGS

BBP-003 Elastomeric Bearing Pads

BHS-007 Railing System Type II

BJE-001 Armored Edge & Neoprene Joints

RBR-001 Steel Beam Guardrail RBR-005 Guardrail Components

SPECIAL NOTES

for Corrosion Inhibitors

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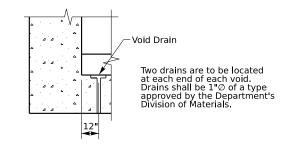
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Division of Structural Design

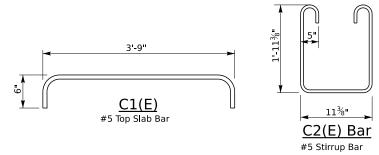
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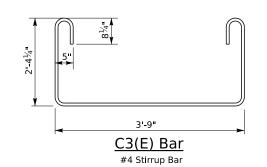
	Strand Data with number indicated in rows													rc	ws		Box Beam Data								C+	raia	Maximum	
	Mark	Midspan Fully Stressed							End Fully Stressed							Total # of	Concrete Stress (psi)		_# of	Approx. Weight	No. of C Bars			s	- Straight Reinforcement			
		1	2						1	2						Strands	f'cı	f'c	Beams	(lbs)	C1	C2	C3		Mark	Size	Length	
	B1	19	17						19	17						36	8000	9000	6	58,263	97	6	97		A1	#5	40'-2"	21/4"
																												i

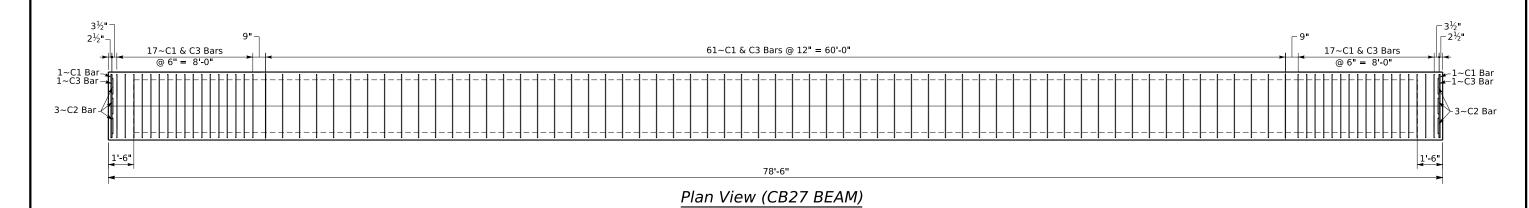
Note: A1 Bars~2 Lengths, 2'-2" Min. Lap 1" clear from ends of beam



VOID DRAIN DETAIL







7~A1 Bars spaced as shown

C2-

 $1\frac{1}{8}$ "cl. for C2

8 spa. @ 2"

CB27 BEAM

4'-0"

 $1\frac{1}{4}$ "cl. for C3

3 spa.

@ 2"

C3 -

_ **℄** *Box*

6"

C2-

3 spa.

@ 2"

 $1\frac{1}{2}$ "cl.

– 1½"cl.

REVISION Division of DATE: October 2024 BOX BEAM CB-27 DETAILS BRIDGE ID 055B00034N CHECKED BY COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS JACKSON DESIGNED BY: L. Likins W. Deaton KY 3630 RAWING NUMBE 28908 Structural Design CROSSING
Sturgeon Creek DETAILED BY: M. BawlThawng W. Deaton DATE PLOTTED: 22-NOV-2024

